

ORDINANCE NO. _____

An ordinance amending Title 30 – Residential Code of the Los Angeles County Code by adopting the 2013 California Residential Code by reference, with certain changes and modifications, and making other revisions thereto.

The Board of Supervisors of the County of Los Angeles ordains as follows:

SECTION XX. Chapters 2 through 10, Chapter 44, and Appendix H, which incorporate by reference and modify portions of the 2010 California Residential Code, are hereby repealed.

SECTION XX. Chapter 1 is hereby amended to read as follows:

R100 ADOPTION BY REFERENCE

Except as hereinafter changed or modified, Sections 102 through 119 of Chapter 1, Section 1207 of Chapter 12, Chapters 34, 67, 69, 98, 99, and Appendix J of Title 26 of the Los Angeles County Code are adopted by reference and incorporated into this Title 30 as if fully set forth below, and shall be known as Sections 102 through 119 of Chapter 1, Section 1207 of Chapter 12, Chapters 34, 67, 69, 98, 99 and Appendix J of Title 30 of the Los Angeles County Code.

Except as hereinafter changed or modified, Chapters 2 through 10, Chapter 44, and Appendix H of that certain code known and designated as the ~~2010~~2013 California Residential Code as published by the California Building Standards Commission are adopted by reference and incorporated into this Title 30 as if fully set forth below, and shall be known as Chapters 2 through 10, Chapter 44, and Appendix H of Title 30 of the Los Angeles County Code.

A copy of the ~~2010~~2013 California Residential Code shall be at all times maintained by the Building Official for use and examination by the public.

SECTION XX. Section R301.1.3.2 is hereby amended to read as follows:

R301.1.3.2 Woodframe structures ~~greater than two stories~~.

The building official shall require construction documents to be stamped by a California licensed architect or engineer for all dwellings of woodframe construction more than two stories and basement in height located in Seismic Design Category A, B or C. Notwithstanding other sections the law, the law establishing these provisions is found in Business and Professions Code Section 5537 and 6737.1.

The building official shall require construction documents to be stamped by a California licensed architect or engineer for all dwellings of woodframe construction more than one story in height or with a basement located in Seismic Design Category D₀, D₁, D₂ or E.

SECTION XX. Section R301.1.4 is hereby added to read as follows:

R301.1.4 Seismic Design Provisions for Buildings Constructed On Or Into Slopes Steeper Than One Unit Vertical In Three Units Horizontal (33.3 Percent Slope).

The design and construction of new buildings and additions to existing buildings when constructed on or into slopes steeper than one unit vertical in three units horizontal (33.3percent slope), shall comply with Section 1613.7 of the 2011 County of Los Angeles Building Code.

SECTION XX. Section R301.2 is hereby amended to read as follows:

R301.2 Climatic and geographic design criteria.

Buildings shall be constructed in accordance with the provisions of this Code as limited by the provisions of this ~~s~~Section. ~~Additional criteria shall be established by the local jurisdiction and set forth~~ Consult with the Building Official regarding additional criteria in Table R301.2(1).

SECTION XX. Section R301.2.2.2.5 is hereby amended to read as follows:

R301.2.2.2.5 Irregular buildings. The seismic provisions of this code shall not be used for irregular structures located in Seismic Design Categories C, D0, D1 and D2.

Irregular portions of structures shall be designed in accordance with accepted engineering practice to the extent the irregular features affect the performance of the remaining structural system. When the forces associated with the irregularity are resisted by a structural system designed in accordance with accepted engineering practice, design of the remainder of the building shall be permitted using the provisions of this code. A building or portion of a building shall be considered to be irregular when one or more of the following conditions occur:

1. When exterior shear wall lines or braced wall panels are not in one plane vertically from the foundation to the uppermost story in which they are required.

Exception: ~~For wood light frame construction, floors with cantilevers or setbacks not exceeding four times the nominal depth of the wood floor joists are~~

~~permitted to support braced wall panels that are out of plane with braced wall panels below provided that:~~

- ~~1. Floor joists are nominal 2 inches by 10 inches (51 mm by 254 mm) or larger and spaced not more than 16 inches (406 mm) on center.~~
- ~~2. The ratio of the back span to the cantilever is at least 2 to 1.~~
- ~~3. Floor joists at ends of braced wall panels are doubled.~~
- ~~4. For wood-frame construction, a continuous rim joist is connected to ends or all cantilever joists. When spliced, the rim joists shall be spliced using a galvanized metal tie not less than 0.058 inch (1.5 mm) (16 gage) and 1 1/2 inches (38 mm) wide fastened with six 16d nails on each side of the splice or a block of the same size as the rim joist of sufficient length to fit securely between the joist space at which the splice occurs fastened with eight 16d nails on each side of the splice; and~~
- ~~5. Gravity loads carried at the end of cantilevered joists are limited to uniform wall and roof loads and the reactions from headers having a span of 18 feet (2438 mm) or less.~~

~~...~~

3. When the end of a braced wall panel occurs over an opening in the wall below and ends at a horizontal distance greater than 1 foot (305 mm) from the edge of the opening. This provision is applicable to shear walls and braced wall panels offset in plane and to braced wall panels offset out of plane as permitted by the exception to item 1 above.

Exception: ~~For wood light-frame wall construction, one end of a braced wall panel shall be permitted to extend more than one foot (305 mm) over an opening not~~

~~more than 8 feet (2438 mm) wide in the wall below provided that the opening includes a header in accordance with the following:~~

~~1. The building width, loading condition and framing member species limitations of Table R502.5(1) shall apply; and~~

~~2. Not less than one 2x12 or two 2x10 for an opening not more than 4 feet (1219 mm) wide; or~~

~~3. Not less than two 2x12 or three 2x10 for an opening not more than 6 feet (1829 mm) wide; or~~

~~4. Not less than three 2x12 or four 2x10 for an opening not more than 8 feet (2438 mm) wide; and~~

~~5. The entire length of the braced wall panel does not occur over an opening in the wall below.~~

~~...~~

~~5. When portions of a floor level are vertically offset.~~

Exceptions:

~~1. Framing supported directly by continuous foundations at the perimeter of the building.~~

~~2. For wood light frame construction, floors shall be permitted to be vertically offset when the floor framing is lapped or tied together as required by section R502.6.1.~~

~~...~~

SECTION XX. Section R301.2.2.3.8 is hereby added to read as follows:

R301.2.2.3.8 Anchorage of Mechanical, Electrical, or Plumbing Components and Equipment.

Mechanical, electrical, or plumbing components and equipment shall be anchored to the structure. Anchorage of the components and equipment shall be designed to resist loads in accordance with the International Building Code and ASCE 7, except where the component is positively attached to the structure and flexible connections are provided between the component and associated ductwork, piping, and conduit; and either

1. The component weighs 400 lb (1,780 N) or less and has a center of mass located 4 ft (1.22 m) or less above the supporting structure; or
2. The component weighs 20 lb (89N) or less or, in the case of a distributed system, 5 lb/ft (73 N/m) or less.

SECTION XX. Table R302.1(2) is hereby amended by deleting footnote a as follows:

TABLE R302.1(2)

**EXTERIOR WALLS—DWELLINGS AND ACCESSORY BUILDINGS WITH
AUTOMATIC RESIDENTIAL FIRE SPRINKLER PROTECTION**

...

- a. ~~For residential subdivisions where all dwellings and accessory buildings are equipped throughout with an automatic sprinkler systems installed in accordance with Section R313, the fire separation distance for nonrated exterior walls and~~

~~rated projections shall be permitted to be reduced to 0 feet, and unlimited unprotected openings and penetrations shall be permitted, where the adjoining lot provides an open setback yard that is 6 feet or more in width on the opposite side of the property line~~

SECTION XX. Section R327.1.1 is hereby amended to read as follows:

R327.1.1 Scope.

This ~~e~~Chapter applies to building materials, systems, and or assemblies used in the exterior design and construction of new buildings ~~located, and to additions, alterations, or repairs made to existing buildings, erected, constructed, or moved~~ within a Wildland-Urban Interface Fire Area as defined in Section R327.2.

SECTION XX. Section R327.1.3 is hereby amended to read as follows:

R327.1.3. Application.

New buildings, and any additions, alterations, or repairs made to existing buildings located in or moved within any Fire Hazard Severity Zone within State Responsibility Areas or any Wildland-Urban Interface Fire Area designated by the ~~enforcing agency~~ Los Angeles County Fire Department constructed after the application date shall comply with the provisions of this ~~e~~Chapter.

Exceptions:

. . .

~~4. Additions to and remodels of buildings originally constructed prior to the applicable application date.~~

SECTION XX. Section R327.1.3.1 is hereby amended to read as follows:

R327.1.3.1 Application date and where required.

New buildings for which an application for a building permit is submitted on or after July 1, 2008, and any additions, alterations, or repairs made to existing buildings for which an application for a building permit is submitted on or after January 1, 2014 located in any Fire Hazard Severity Zone or Wildland Interface Fire Area shall comply with all ~~s~~Sections of this ~~e~~Chapter, including all of the following areas:

...

Exceptions:

1. ~~New~~ bBuildings located in any Fire Hazard Severity Zone within State Responsibility Areas, for which an application for a building permit is submitted on or after January 1, 2008, shall comply with all ~~s~~Sections of this ~~e~~Chapter.

2. ~~New~~ bBuildings located in any Fire Hazard Severity Zone within State Responsibility Areas or any Wildland Interface Fire Area designated by cities and other local agencies for which an application for a building permit is submitted on or after December 1, 2005, but prior to July 1, 2008, shall only comply with the following ~~s~~Sections of this ~~e~~Chapter:

...

SECTION XX. Section R327.1.4 is hereby amended to read as follows:

R327.1.4 Inspection and certification.

...

1. Building permit issuance. The ~~local~~ bBuilding ~~e~~Official shall, prior to construction, provide the owner or applicant a certification that the building as proposed to be built complies with all applicable state and local building standards, including those for materials and construction methods for wildfire exposure as described in this ~~e~~Chapter. Issuance of a building permit by the ~~local~~ bBuilding ~~e~~Official for the proposed building shall be considered as complying with this ~~s~~Section.

2. Building permit final. The local Building Official shall, upon completion of construction, provide the owner or applicant with a copy of the final inspection report that demonstrates the building was constructed in compliance with all applicable state and local building standards, including those for materials and construction methods for wildfire exposure as described in this Chapter. Issuance of a certificate of occupancy by the local Building Official for the proposed building shall be considered as complying with this Section.

SECTION XX.

Section R327.2 is hereby amended to read as follows:

SECTION R327.2

DEFINITIONS

...

FIRE PROTECTION PLAN is a document prepared for a specific project or development proposed for a Wildland-Urban Interface Fire Area. It describes ways to minimize and mitigate potential for loss from wildfire exposure.

The fire protection plan shall be in accordance with this Chapter and the California Los Angeles County Fire Code Title 32, Chapter 49. When required by the enforcing agency for the purposes of granting modifications, a fire protection plan shall be submitted. ~~Only locally adopted ordinances that have been filed with the California Building Standards Commission or the Department of Housing and Community Development in accordance with Section 1.1.8 shall apply.~~

FIRE HAZARD SEVERITY ZONES are geographical areas designated pursuant to California Public Resources Code Sections 4201 through 4204 and classified as Very High, High, or Moderate in State Responsibility Areas or as Local Agency Very High Fire Hazard

Severity Zones designated pursuant to California Government Code sections 51175 through 51189. See ~~California~~Los Angeles County Fire Code, Article 86 Chapter 49.

...

HEAVY TIMBER. A type of construction classification specified in Section 602. For use in this Chapter, heavy timber shall be sawn lumber or glue laminated wood with the smallest minimum nominal dimension of 4 inches (102 mm). Heavy Timber walls or floors shall be sawn or glue-laminated planks splined, tongue-and-grove, or set close together and well spiked.

...

WILDLAND-URBAN INTERFACE FIRE AREA is a geographical area identified by the state as a "Fire Hazard Severity Zone" in accordance with the Public Resources Code Sections 4201 through 4204 and Government Code Sections 51175 through 51189, or other areas designated by the ~~enforcing agency~~Los Angeles County Fire Department to be at a significant risk from wildfires.

SECTION XX. Section R327.3.2 is hereby amended to read as follows:

R327.3.2 Qualification by testing.

Material and material assemblies tested in accordance with the requirements of Section R327.3 shall be accepted for use when the results and conditions of those tests are met. Product evaluation testing of material and material assemblies shall be approved or listed by the State Fire Marshal, the Building Official or identified in a current report issued by an approved agency.

SECTION XX. Section R327.3.3 is hereby amended to read as follows:

R327.3.3 Approved agency.

Product evaluation testing shall be performed by an approved agency as defined in Section 1702. The scope of accreditation for the approved agency shall include building product compliance with eCode.

SECTION XX. Section R327.3.5.2 is hereby amended to read as follows:

R327.3.5.2 Weathering.

Fire-retardant-treated wood and fire-retardant-treated wood shingles and shakes shall meet the fire test performance requirements of this eChapter after being subjected to the weathering conditions contained in the following standards, as applicable to the materials and the conditions of use.

SECTION XX. Section R327.3.5.2.1 is hereby amended to read as follows:

R327.3.5.2.1 Fire-retardant-treated wood.

Fire-retardant-treated wood shall be tested in accordance with ASTM D2898, "Standard Practice for Accelerated Weathering of Fire-Retardant Treated Wood for Fire Testing (Method A)" and the requirements of Section 2303.2.

SECTION XX. Section R327.3.5.2.2 is hereby deleted in its entirety.

~~**R327.3.5.2.2 Fire-retardant-treated wood shingles and shakes.** Fire-retardant-treated wood shingles and shakes shall be approved and listed by the State Fire marshal in accordance with Section 208(c), Title 19 California Code of Regulations.~~

SECTION XX. Section R327.3.6 is hereby amended to read as follows:

R327.3.6 Alternates for materials, design, tests, and methods of construction.

The enforcing agency is permitted to modify the provisions of this eChapter for site-specific conditions in accordance with Chapter 1, Section 4.11.2.4104.2.7. When required by the ~~enforcing agency~~ Building Official for the purposes of granting modifications, a fire

protection plan shall be submitted in accordance with the ~~California~~Los Angeles County Fire Code, Chapter 49.

SECTION XX. Section R327.4.3 is hereby amended to read as follows:

R327.4.3 Alternative methods for determining Ignition-resistant material.

...

2. Fire-retardant-treated wood. Fire-retardant-treated wood identified for exterior use that complies with the requirements of Section 2303.2 of the ~~California~~Los Angeles County Building Code.

~~3. Fire-retardant-treated wood shingles and shakes. Fire-retardant-treated wood shingles and shakes, as defined in section 1505.6 and listed by State Fire Marshal for use as "Class B" roof covering, shall be accepted as an Ignition-resistant wall covering material when installed over solid sheathing.~~

SECTION XX. Section R327.5.2. is hereby amended to read as follows:

R327.5.2 Roof coverings.

Roof coverings shall be Class A as specified in Section R902.1. Where the roof profile allows a space between the roof covering and roof decking, the spaces shall be constructed to prevent the intrusion of flames and embers, be firestopped with approved materials or have one layer of minimum 72 pounds (32.4 kg) mineral-surfaced non-perforated cap sheet complying with ASTM D 3909 installed over the combustible decking. Wood shingles and wood shakes are prohibited in any Fire Hazard Severity Zones regardless of classification.

SECTION XX. Section R327.6.1 is hereby amended to read as follows:

R327.6.1 General.

Where provided, ventilation openings for enclosed attics, enclosed eave soffit spaces, enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters, and underfloor ventilation shall be in accordance with Section 1203 of the ~~California~~Los Angeles County Building Code and Sections R327.6.1 through R327.6.3 of this ~~s~~Section to resist building ignition from the intrusion of burning embers and flame through the ventilation openings.

SECTION XX. Section R327.6.3 is hereby amended to read as follows:

...

Exceptions:

1. The ~~enforcing agency~~Building Official may accept or approve special eave and cornice vents that resist the intrusion of flame and burning embers.

2. Vents complying with the requirements of Section R327.6.2 may be installed on the underside of eaves and cornices in accordance with either one of the following conditions:

2.1. The attic space being ventilated is fully protected by an automatic sprinkler system installed in accordance with Section 903.3.1.1 of the ~~California~~Los Angeles County Building Code or,

...

SECTION XX. Section R327.10.2 is hereby amended to read as follows:

R327.10.2 Applicability.

...

Exceptions:

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2. Awnings and canopies shall comply with the requirements of Section 3105.

SECTION XX. Section R327.10.3.2 is hereby amended to read as follows:

R327.10.3.2 When required by the ~~enforcing agency~~Building Official, detached accessory structures within 50 feet of an applicable building shall comply with the requirements of this ~~s~~Section.

SECTION XX. Section R327.10.4 is hereby amended to read as follows:

R327.10.4. Requirements.

When required by the ~~enforcing agency~~Building Official, accessory structures shall be constructed of noncombustible or ignition-resistant materials.

SECTION XX. Section R401.1 is hereby amended to read as follows:

R401.1 Application.

...

Wood foundations in Seismic Design Category D₀, D₁ or D₂ shall ~~be designed in accordance with accepted engineering practice~~not be permitted.

Exception: In non-occupied, single-story, detached storage sheds and similar uses other than carport or garage, provided the gross floor area does not exceed 200 square feet, the plate height does not exceed 12 feet in height above the grade plane at any point, and the maximum roof projection does not exceed 24 inches.

SECTION XX. Section R403.1.2 is hereby amended to read as follows:

R403.1.2 Continuous footing in Seismic Design Categories D₀, D₁, and D₂.

The braced wall panels at exterior walls of buildings located in Seismic Design Categories D_0 , D_1 , and D_2 shall be supported by continuous footings. All required interior braced wall panels in buildings ~~with plan dimensions greater than 50 feet (15240 mm)~~ shall also be supported by continuous footings.

SECTION XX. Section R403.1.3 is hereby amended to read as follows:

R403.1.3 Seismic reinforcing.

...

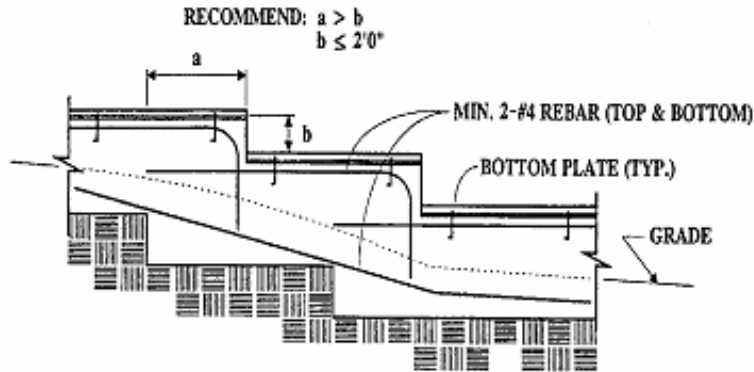
Exception: In detached one- and two-family dwellings located in Seismic Design Category A, B or C which are three stories or less in height and constructed with stud bearing walls, isolated plain concrete footings, supporting columns or pedestals are permitted.

SECTION XX. Section R403.1.5 is hereby amended to read as follows:

R403.1.5 Slope.

The top surface of footings shall be level. The bottom surface of footings shall be permitted to have a slope not exceeding one unit vertical in 10 units horizontal (10-percent slope). Footings shall be stepped where it is necessary to change the elevation of the top surface of the footing or where the surface of the ground slopes more than one unit vertical in 10 units horizontal (10-percent slope).

For structures located in Seismic Design Categories D_0 , D_1 , or D_2 , stepped footings shall be reinforced with two No. 4 deformed reinforcing bars located at the top and bottom of the footings as shown in Figure R403.1.5.



STEPPED FOUNDATIONS

FIGURE R403.1.5

STEPPED FOOTING

SECTION XX. Section R404.2 is hereby amended to read as follows:

R404.2 Wood foundation walls.

Wood foundation walls shall be constructed in accordance with the provisions of Sections R404.2.1 through R404.2.6 and with the details shown in Figures R403.1(2) and R403.1(3). Wood foundation walls shall not be used for structures located in Seismic Design Category D₀, D₁, or D₂.

SECTION XX. Section R501.1 is hereby amended to read as follows:

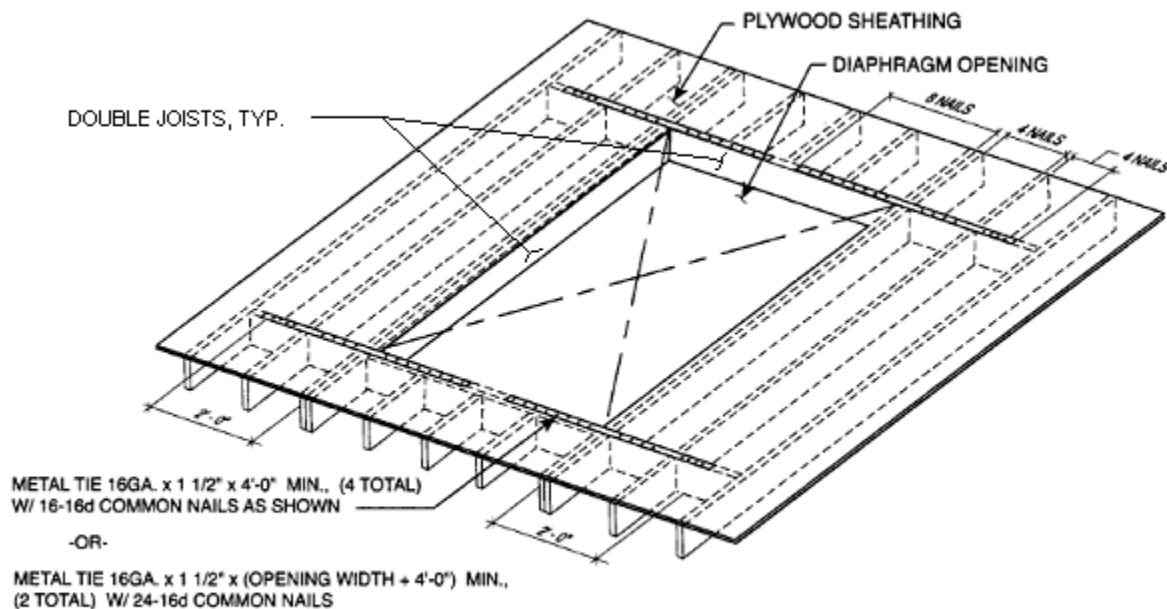
R501.1 Application.

The provisions of this chapter shall control the design and construction of the floors for all buildings including the floors of attic spaces used to house mechanical or plumbing fixtures and equipment. Mechanical or plumbing fixtures and equipment shall be attached (or anchored) to the structure in accordance with Section R301.2.2.3.8

SECTION XX. Section R503.2.4 is hereby added to read as follows:

R503.2.4 Openings in horizontal diaphragms.

Openings in horizontal diaphragms with a dimension perpendicular to the joist that is greater than 4 feet (1.2 m) shall be constructed in accordance with Figure R503.2.4.



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

- a. Blockings shall be provided beyond headers.
- b. Metal ties not less than 0.058 inch [1.47 mm (16 galvanized gage)] by 1.5 inches (38 mm) wide with eight 16d common nails on each side of the header-joist intersection. The metal ties shall have a minimum yield of 33,000 psi (227 MPa).
- c. Openings in diaphragms shall be further limited in accordance with Section R301.2.2.2.5.

Figure R503.2.4

SECTION XX. Section R602.3.2 is hereby amended to read as follows:

R602.3.2**Top plate.** Wood stud walls shall be capped with a double

top plate installed to provide overlapping at corners and intersections with bearing partitions. End joints in top plates shall be offset at least 24 inches (610 mm). Joints in plates need not occur over studs. Plates shall be not less than 2-inches (51 mm) nominal thickness and have a width at least equal to the width of the studs.

Exception: In other than Seismic Design Category D₀, D₁ or D₂, a A single top plate may be installed in stud walls, provided the plate is adequately tied at joints, corners and intersecting walls by a minimum 3-inch-by-6-inch by a 0.036-inch-thick (76 mm by 152 mm by 0.914 mm) galvanized steel plate that is nailed to each wall or segment of wall by six 8d nails on each side, provided the rafters or joists are centered over the studs with a tolerance of no more than 1 inch (25 mm). The top plate may be omitted over lintels that are adequately tied to adjacent wall sections with steel plates or equivalent as previously described.

SECTION XX. Table R602.3(1) is hereby amended to read as follows:

TABLE R602.3(1)**FASTENER SCHEDULE FOR STRUCTURAL MEMBERS**

TABLE R602.3(1)
FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

ITEM	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER ^{a, b, c}	SPACING OF FASTENERS
Roof			
1	Blocking between joists or rafters to top plate, toe nail	3-8d (2½" × 0.113")	—
2	Ceiling joists to plate, toe nail	3-8d (2½" × 0.113")	—
3	Ceiling joists not attached to parallel rafter, laps over partitions, face nail	3-10d	—
4	Collar tie to rafter, face nail or 1¼" × 20 gage ridge strap	3-10d (3" × 0.128")	—
5	Rafter or roof truss to plate, toe nail	3-16d box nails (3½" × 0.135") or 3-10d common nails (3" × 0.148")	2 toe nails on one side and 1 toe nail on opposite side of each rafter or truss ^j
6	Roof rafters to ridge, valley or hip rafters: toe nail face nail	4-16d (3½" × 0.135") 3-16d (3½" × 0.135")	—
Wall			
7	Built-up studs-face nail	10d (3" × 0.128")	24" o.c.
8	Abutting studs at intersecting wall corners, face nail	16d (3½" × 0.135")	12" o.c.
9	Built-up header, two pieces with ½" spacer	16d (3½" × 0.135")	16" o.c. along each edge
10	Continued header, two pieces	16d (3½" × 0.135")	16" o.c. along each edge
11	Continuous header to stud, toe nail	4-8d (2½" × 0.113")	—
12	Double studs, face nail	10d (3" × 0.128")	24" o.c.
13	Double top plates, face nail	10d (3" × 0.128")	24" o.c.
14	Double top plates, minimum 24-inch offset of end joints, face nail in lapped area	8-16d (3½" × 0.135")	—
15	Sole plate to joist or blocking, face nail	16d (3½" × 0.135")	16" o.c.
16	Sole plate to joist or blocking at braced wall panels	3-16d (3½" × 0.135")	16" o.c.
17	Stud to sole plate, toe nail	3-8d (2½" × 0.113") or 2-16d (3½" × 0.135")	— —
18	Top or sole plate to stud, end nail	2-16d (3½" × 0.135")	—
19	Top plates, laps at corners and intersections, face nail	2-10d (3" × 0.128")	—
20	1" brace to each stud and plate, face nail	2-8d (2½" × 0.113") 2 staples 1¾"	— —
21	1" × 6" sheathing to each bearing, face nail	2-8d (2½" × 0.113") 2 staples 1¾"	— —
22	1" × 8" sheathing to each bearing, face nail	2-8d (2½" × 0.113") 3 staples 1¾"	— —
23	Wider than 1" × 8" sheathing to each bearing, face nail	3-8d (2½" × 0.113") 4 staples 1¾"	— —
Floor			
24	Joist to sill or girder, toe nail	3-8d (2½" × 0.113")	—
25	Rim joist to top plate, toe nail (roof applications also)	8d (2½" × 0.113")	6" o.c.
26	Rim joist or blocking to sill plate, toe nail	8d (2½" × 0.113")	6" o.c.
27	1" × 6" subfloor or less to each joist, face nail	2-8d (2½" × 0.113") 2 staples 1¾"	— —
28	2" subfloor to joist or girder, blind and face nail	2-16d (3½" × 0.135")	—
29	2" planks (plank & beam - floor & roof)	2-16d (3½" × 0.135")	at each bearing
30	Built-up girders and beams, 2-inch lumber layers	10d (3" × 0.128")	Nail each layer as follows: 32" o.c. at top and bottom and staggered. Two nails at ends and at each splice.
31	Ledger strip supporting joists or rafters	3-16d (3½" × 0.135")	At each joist or rafter

(continued)

TABLE R602.3(1)—continued
FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

ITEM	DESCRIPTION OF BUILDING MATERIALS	DESCRIPTION OF FASTENER ^{b, c, e}	SPACING OF FASTENERS	
			Edges (inches) ^f	Intermediate supports ^{c, e} (inches)
Wood structural panels, subfloor, roof and interior wall sheathing to framing and particleboard wall sheathing to framing				
32	$\frac{3}{8}$ " - $\frac{1}{2}$ "	6d common (2" × 0.113") nail (subfloor wall) ^j 8d common (2 $\frac{1}{2}$ " × 0.131") nail (roof) ^f	6	12 ^g
33	$\frac{19}{32}$ " - 1"	8d common nail (2 $\frac{1}{2}$ " × 0.131")	6	12 ^g
34	1 $\frac{1}{8}$ " - 1 $\frac{1}{4}$ "	10d common (3" × 0.148") nail or 8d (2 $\frac{1}{2}$ " × 0.131") deformed nail	6	12
Other wall sheathing ^h				
35	$\frac{1}{2}$ " structural cellulosic fiberboard sheathing	1 $\frac{1}{2}$ " galvanized roofing nail, $\frac{7}{16}$ " crown or 1" crown staple 16 ga., 1 $\frac{1}{4}$ " long	3	6
36	$\frac{25}{32}$ " structural cellulosic fiberboard sheathing	1 $\frac{3}{4}$ " galvanized roofing nail, $\frac{7}{16}$ " crown or 1" crown staple 16 ga., 1 $\frac{1}{2}$ " long	3	6
37 ^k	$\frac{1}{2}$ " gypsum sheathing ^d	1 $\frac{1}{2}$ " galvanized roofing nail; staple galvanized, 1 $\frac{1}{2}$ " long; 1 $\frac{1}{4}$ " screws, Type W or S	7	7
38 ^k	$\frac{5}{8}$ " gypsum sheathing ^d	1 $\frac{3}{4}$ " galvanized roofing nail; staple galvanized, 1 $\frac{5}{8}$ " long; 1 $\frac{5}{8}$ " screws, Type W or S	7	7
Wood structural panels, combination subfloor underlayment to framing				
39	$\frac{3}{4}$ " and less	6d deformed (2" × 0.120") nail or 8d common (2 $\frac{1}{2}$ " × 0.131") nail	6	12
40	$\frac{7}{8}$ " - 1"	8d common (2 $\frac{1}{2}$ " × 0.131") nail or 8d deformed (2 $\frac{1}{2}$ " × 0.120") nail	6	12
41	1 $\frac{1}{8}$ " - 1 $\frac{1}{4}$ "	10d common (3" × 0.148") nail or 8d deformed (2 $\frac{1}{2}$ " × 0.120") nail	6	12

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 Ksi = 6.895 MPa.

- a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi for shank diameter of 0.192 inch (20d common nail), 90 ksi for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi for shank diameters of 0.142 inch or less.
- b. Staples are 16 gage wire and have a minimum ⅞-inch on diameter crown width.
- c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater.
- d. Four-foot by 8-foot or 4-foot by 9-foot panels shall be applied vertically.
- e. Spacing of fasteners not included in this table shall be based on Table R602.3(2).
- f. For regions having basic wind speed of 110 mph or greater, 8d deformed (2½" × 0.120") nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable end walls, if mean roof height is more than 25 feet, up to 35 feet maximum.
- g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable end wall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.
- h. Gypsum sheathing shall conform to ASTM C 1396 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208.
- i. Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and required blocking and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and required blocking. Blocking of roof or floor sheathing panel edges perpendicular to the framing members need not be provided except as required by other provisions of this code. Floor perimeter shall be supported by framing members or solid blocking.
- j. Where a rafter is fastened to an adjacent parallel ceiling joist in accordance with this schedule, provide two toe nails on one side of the rafter and toe nails from the ceiling joist to top plate in accordance with this schedule. The toe nail on the opposite side of the rafter shall not be required.

K. Use of staples in braced wall panels shall be prohibited in Seismic Design Category D0, D1, or D2.

SECTION XX. Table R602.3(2) is hereby amended to read as follows:

TABLE R602.3(2)

ALTERNATE ATTACHMENTS TO TABLE R602.3(1)

...

- b. Staples shall have a minimum crown width of 7/16-inch on diameter except as noted. Use of staples in roof, floor, subfloor, and braced wall panels shall be prohibited in Seismic Design Category D₀, D₁, or D₂.







...

SECTION XX. Table R602.10.3(3) is hereby amended to read as follows:

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TABLE R602.10.3(3)

BRACING REQUIREMENTS BASED ON SEISMIC DESIGN CATEGORY

<ul style="list-style-type: none"> • SOIL CLASS D^b • WALL HEIGHT = 10 FEET • 10 PSF FLOOR DEAD LOAD • 15 PSF ROOF/CEILING DEAD LOAD • BRACED WALL LINE SPACING ≤ 25 FEET 			TABLE R602.10.3(3) BRACING REQUIREMENTS BASED ON SEISMIC DESIGN CATEGORY MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE ^a				
Seismic Design Category	Story Location	Braced Wall Line Length (feet)	Method LIB ^c	Method GB [±]	Methods DWB, SFB, PBS, PCP, HPS, CS-SFB ^{d,±}	Method WSP	Methods CS-WSP, CS-G
C (townhouses only)		10	2.5	2.5	2.5	1.6	1.4
		20	5.0	5.0	5.0	3.2	2.7
		30	7.5	7.5	7.5	4.8	4.1
		40	10.0	10.0	10.0	6.4	5.4
		50	12.5	12.5	12.5	8.0	6.8
		10	NP	4.5	4.5	3.0	2.6
		20	NP	9.0	9.0	6.0	5.1
		30	NP	13.5	13.5	9.0	7.7
		40	NP	18.0	18.0	12.0	10.2
		50	NP	22.5	22.5	15.0	12.8
		10	NP	6.0	6.0	4.5	3.8
		20	NP	12.0	12.0	9.0	7.7
		30	NP	18.0	18.0	13.5	11.5
		40	NP	24.0	24.0	18.0	15.3
		50	NP	30.0	30.0	22.5	19.1
D ₀		10	NP	2.8 <u>5.6</u>	2.8 <u>5.6</u>	1.8	1.6
		20	NP	5.5 <u>11.0</u>	5.5 <u>11.0</u>	3.6	3.1
		30	NP	8.3 <u>16.6</u>	8.3 <u>16.6</u>	5.4	4.6
		40	NP	11.0 <u>22.0</u>	11.0 <u>22.0</u>	7.2	6.1
		50	NP	13.8 <u>27.6</u>	13.8 <u>27.6</u>	9.0	7.7
		10	NP	5.3 <u>NP</u>	5.3 <u>NP</u>	3.8	3.2
		20	NP	10.5 <u>NP</u>	10.5 <u>NP</u>	7.5	6.4
		30	NP	15.8 <u>NP</u>	15.8 <u>NP</u>	11.3	9.6
		40	NP	21.0 <u>NP</u>	21.0 <u>NP</u>	15.0	12.8
		50	NP	26.3 <u>NP</u>	26.3 <u>NP</u>	18.8	16.0
		10	NP	7.3 <u>NP</u>	7.3 <u>NP</u>	5.3	4.5
		20	NP	14.5 <u>NP</u>	14.5 <u>NP</u>	10.5	9.0
		30	NP	21.8 <u>NP</u>	21.8 <u>NP</u>	15.8	13.4
		40	NP	29.0 <u>NP</u>	29.0 <u>NP</u>	21.0	17.9
		50	NP	36.3 <u>NP</u>	36.3 <u>NP</u>	26.3	22.3

(continued)

TABLE R602.10.3(3)—continued
BRACING REQUIREMENTS BASED ON SEISMIC DESIGN CATEGORY

<ul style="list-style-type: none"> • SOIL CLASS D^b • WALL HEIGHT = 10 FEET • 10 PSF FLOOR DEAD LOAD • 15 PSF ROOF/CEILING DEAD LOAD • BRACED WALL LINE SPACING ≤ 25 FEET 			MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE ^c				
Seismic Design Category	Story Location	Braced Wall Line Length (feet)	Method LIB ^d	Method GB ^e	Methods DWB, SFB, PBS, PCP, HPS, CS-SFB ^{d,e}	Method WSP	Methods CS-WSP, CS-G
D ₁		10	NP	3.0 6.0	3.0 6.0	2.0	1.7
		20	NP	6.0 12.0	6.0 12.0	4.0	3.4
		30	NP	9.0 18.0	9.0 18.0	6.0	5.1
		40	NP	12.0 24.0	12.0 24.0	8.0	6.8
		50	NP	15.0 30.0	15.0 30.0	10.0	8.5
		10	NP	6.0 NP	6.0 NP	4.5	3.8
		20	NP	12.0 NP	12.0 NP	9.0	7.7
		30	NP	18.0 NP	18.0 NP	13.5	11.5
		40	NP	24.0 NP	24.0 NP	18.0	15.3
		50	NP	30.0 NP	30.0 NP	22.5	19.1
		10	NP	8.5 NP	8.5 NP	6.0	5.1
		20	NP	17.0 NP	17.0 NP	12.0	10.2
		30	NP	25.5 NP	25.5 NP	18.0	15.3
		40	NP	34.0 NP	34.0 NP	24.0	20.4
		50	NP	42.5 NP	42.5 NP	30.0	25.5
D ₂		10	NP	4.0 8.0	4.0 8.0	2.5	2.1
		20	NP	8.0 16.0	8.0 16.0	5.0	4.3
		30	NP	12.0 24.0	12.0 24.0	7.5	6.4
		40	NP	16.0 32.0	16.0 32.0	10.0	8.5
		50	NP	20.0 40.0	20.0 40.0	12.5	10.6
		10	NP	7.5 NP	7.5 NP	5.5	4.7
		20	NP	15.0 NP	15.0 NP	11.0	9.4
		30	NP	22.5 NP	22.5 NP	16.5	14.0
		40	NP	30.0 NP	30.0 NP	22.0	18.7
		50	NP	37.5 NP	37.5 NP	27.5	23.4
		10	NP	NP	NP	NP	NP
		20	NP	NP	NP	NP	NP
		30	NP	NP	NP	NP	NP
		40	NP	NP	NP	NP	NP
		50	NP	NP	NP	NP	NP
	Cripple wall below one- or two-story dwelling	10	NP	NP	NP	7.5	6.4
		20	NP	NP	NP	15.0	12.8
		30	NP	NP	NP	22.5	19.1
		40	NP	NP	NP	30.0	25.5
		50	NP	NP	NP	37.5	31.9

For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 pound per square foot = 0.0479 kPa.

a. Linear interpolation shall be permitted.

b. Wall bracing lengths are based on a soil site class "D." Interpolation of bracing length between the S_{ds} values associated with the Seismic Design Categories shall be permitted when a site-specific S_{ds} value is determined in accordance with Section 1613.3 of the *International Building Code*.

c. Method LIB shall have gypsum board fastened to at least one side with nails or screws per Table R602.3(1) for exterior sheathing or Table R702.3.5 for interior gypsum board. Spacing of fasteners at panel edges shall not exceed 8 inches.



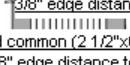






d. Method CS-SFB applies in SDC C only.

e. Methods GB and PCP braced wall panel h/w ratio shall not exceed 1:1 in SDC D0, D1 or D2. Methods DWB, SFB, PBS, and HPS are not permitted in SDC D0, D1, or D2.

SECTION XX. Table R602.10.4 is hereby amended to read as follows:

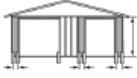

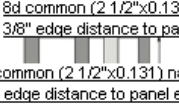



TABLE R602.10.4
BRACING METHODS

TABLE R602.10.4
BRACING METHODS ¹

METHODS, MATERIAL		MINIMUM THICKNESS	FIGURE	CONNECTION CRITERIA ^a	
				Fasteners	Spacing
Intermittent Bracing Method	LIB Let-in-bracing	1 × 4 wood or approved metal straps at 45° to 60° angles for maximum 16" stud spacing		Wood: 2-8d common nails or 3-8d (2½" long x 0.113" dia.) nails Metal strap: per manufacturer	Wood: per stud and top and bottom plates Metal: per manufacturer
	DWB Diagonal wood boards	¾"(1" nominal) for maximum 24" stud spacing		2-8d (2½" long x 0.113" dia.) nails or 2 - 1¾" long staples	Per stud
	WSP Wood structural panel (See Section R604)	$\frac{3}{8}"$ <u>15/32"</u>	 8d common (2 1/2"x0.131) nails 3/8" edge distance to panel edge	Exterior sheathing per Table R602.3(3) Interior sheathing per Table R602.3(1) or R602.3(2)	6" edges 12" field Varies by fastener 6" edges 12" field
	BV-WSP^a Wood Structural Panels with Stone or Masonry Veneer (See Section R602.10.6.5)	7/16"	See Figure R602.10.6.5	8d common (2½" × 0.131) nails	4" at panel edges 12" at intermediate supports 4" at braced wall panel end posts
	SFB Structural fiberboard sheathing	½" or 25/32" for maximum 16" stud spacing		1½" long × 0.12" dia. (for ½" thick sheathing) 1¾" long × 0.12" dia. (for 25/32" thick sheathing) galvanized roofing nails or 8d common (2½" long × 0.131" dia.) nails	3" edges 6" field
	GB Gypsum board	½"		Nails or screws per Table R602.3(1) for exterior locations Nails or screws per Table R702.3.5 for interior locations	For all braced wall panel locations: 7" edges (including top and bottom plates) 7" field
	PBS Particleboard sheathing (See Section R605)	3/8" or ½" for maximum 16" stud spacing		For 3/8", 6d common (2" long × 0.113" dia.) nails For ½", 8d common (2½" long × 0.131" dia.) nails	3" edges 6" field
	PCP Portland cement plaster	See Section R703.6 for maximum 16" stud spacing		1½" long, 11 gage, 7/16" dia. head nails or 7/8" long, 16 gage staples ^a	6" o.c. on all framing members
	HPS Hardboard panel siding	7/16" for maximum 16" stud spacing		0.092" dia., 0.225" dia. head nails with length to accommodate 1½" penetration into studs	4" edges 8" field
	ABW Alternate braced wall	3/8"		See Section R602.10.6.1	See Section R602.10.6.1

(continued)

**TABLE R602.10.4—continued
BRACING METHODS^f**

METHODS, MATERIAL		MINIMUM THICKNESS	FIGURE	CONNECTION CRITERIA ^g	
				Fasteners	Spacing
Intermittent Bracing Methods	PFH Portal frame with hold-downs	$\frac{3}{8}$ "		See Section R602.10.6.2	See Section R602.10.6.2
	PFG Portal frame at garage	$\frac{7}{16}$ "		See Section R602.10.6.3	See Section R602.10.6.3
Continuous Sheathing Methods	CS-WSP Continuously sheathed wood structural panel	$\frac{3}{8}$ " $\frac{15}{32}$ "	 8d common (2 1/2"x0.131") nails 3/8" edge distance to panel edge	Exterior sheathing per Table R602.3(3) Interior sheathing per Table R602.3(1) or R602.3(2)	6" edges 12" field Varies by fastener 6" edges 12" field
	CS-G^{a,c} Continuously sheathed wood structural panel adjacent to garage openings	$\frac{3}{8}$ " $\frac{15}{32}$ "		See Method CS-WSP	See Method CS-WSP
	CS-PF Continuously sheathed portal frame	$\frac{7}{16}$ " $\frac{15}{32}$ "		See Section R602.10.6.4	See Section R602.10.6.4
	CS-SFB^d Continuously sheathed structural fiberboard	$\frac{1}{2}$ " or $\frac{25}{32}$ " for maximum 16" stud spacing		$1\frac{1}{2}$ " long \times 0.12" dia. (for $\frac{1}{2}$ " thick sheathing) $1\frac{3}{4}$ " long \times 0.12" dia. (for $\frac{25}{32}$ " thick sheathing) galvanized roofing nails or 8d common (2 1/2" long \times 0.131" dia.) nails	3" edges 6" field

For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 degree = 0.0175 rad, 1 pound per square foot = 47.8 N/m², 1 mile per hour = 0.447 m/s.

a. Adhesive attachment of wall sheathing, including Method GB, shall not be permitted in Seismic Design Categories C, D₀, D₁ and D₂.

b. Applies to panels next to garage door opening when supporting gable end wall or roof load only. May only be used on one wall of the garage. In Seismic Design Categories D₀, D₁ and D₂ roof covering dead load may not exceed 3 psf.

c. Garage openings adjacent to a Method CS-G panel shall be provided with a header in accordance with Table R502.5(1). A full height clear opening shall not be permitted adjacent to a Method CS-G panel.

d. Method CS-SFB does not apply in Seismic Design Categories D₀, D₁ and D₂ and in areas where the wind speed exceeds 100 mph.

e. Method applies to detached one- and two-family dwellings in Seismic Design Categories D₀ through D₂ only.

f. Methods GB and PCP braced wall panel h/w ratio shall not exceed 1:1 in SDC D0, D1, or D2. Methods LIB, DWB, SFB, PBS, HPS, and PFG are not permitted in SDC D0, D1, or D2.

g. Use of staples in braced wall panels shall be prohibited in SDC D0, D1, or D2.

SECTION XX. Table R602.10.5 is hereby amended to read as follows:

TABLE R602.10.5

MINIMUM LENGTH OF BRACED WALL PANELS

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**TABLE R602.10.5
MINIMUM LENGTH OF BRACED WALL PANELS**

METHOD (See Table R602.10.4)		MINIMUM LENGTH ^a (inches)					CONTRIBUTING LENGTH (inches)
		Wall Height					
		8 feet	9 feet	10 feet	11 feet	12 feet	
DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP		48	48	48	53	58	Actual ^b
GB		48	48	48	53	58	Double sided = Actual Single sided = 0.5 × Actual
LIB		55	62	69	NP	NP	Actual ^b
ABW	SDC A, B and C, wind speed < 110 mph	28	32	34	38	42	48
	SDC D ₀ , D ₁ and D ₂ , wind speed < 110 mph	32	32	34	NP	NP	
PFH	Supporting roof only	16 24	18 24	16 24	18 24 ^c	20 24 ^c	48
	Supporting one story and roof	24	24	24	27 ^c	29 ^c	48
PFG		24	27	30	33 ^d	36 ^d	1.5 × Actual ^b
CS-G		24	27	30	33	36	Actual ^b
CS-PF		16 24	18 24	20 24	22 24 ^e	24 ^e	Actual ^b
CS-WSP, CS-SFB	Adjacent clear opening height (inches)						Actual ^b
	≤ 64	24	27	30	33	36	
	68	26	27	30	33	36	
	72	27	27	30	33	36	
	76	30	29	30	33	36	
	80	32	30	30	33	36	
	84	35	32	32	33	36	
	88	38	35	33	33	36	
	92	43	37	35	35	36	
	96	48	41	38	36	36	
	100	—	44	40	38	38	
	104	—	49	43	40	39	
	108	—	54	46	43	41	
	112	—	—	50	45	43	
	116	—	—	55	48	45	
	120	—	—	60	52	48	
	124	—	—	—	56	51	
	128	—	—	—	61	54	
	132	—	—	—	66	58	
	136	—	—	—	—	62	
	140	—	—	—	—	66	
	144	—	—	—	—	72	

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s.

NP = Not Permitted.

a. Linear interpolation shall be permitted.

b. Use the actual length when it is greater than or equal to the minimum length.

c. Maximum header height for PFH is 10 feet in accordance with Figure R602.10.6.2, but wall height may be increased to 12 feet with pony wall.

d. Maximum opening height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height may be increased to 12 feet with pony wall.

e. Maximum opening height for CS-PF is 10 feet in accordance with Figure R602.10.6.4, but wall height may be increased to 12 feet with pony wall.

SECTION XX. Figure R602.10.6.1 is amended to read as follows:

FIGURE R602.10.6.1

METHOD ABW – ALTERNATE BRACED WALL PANEL

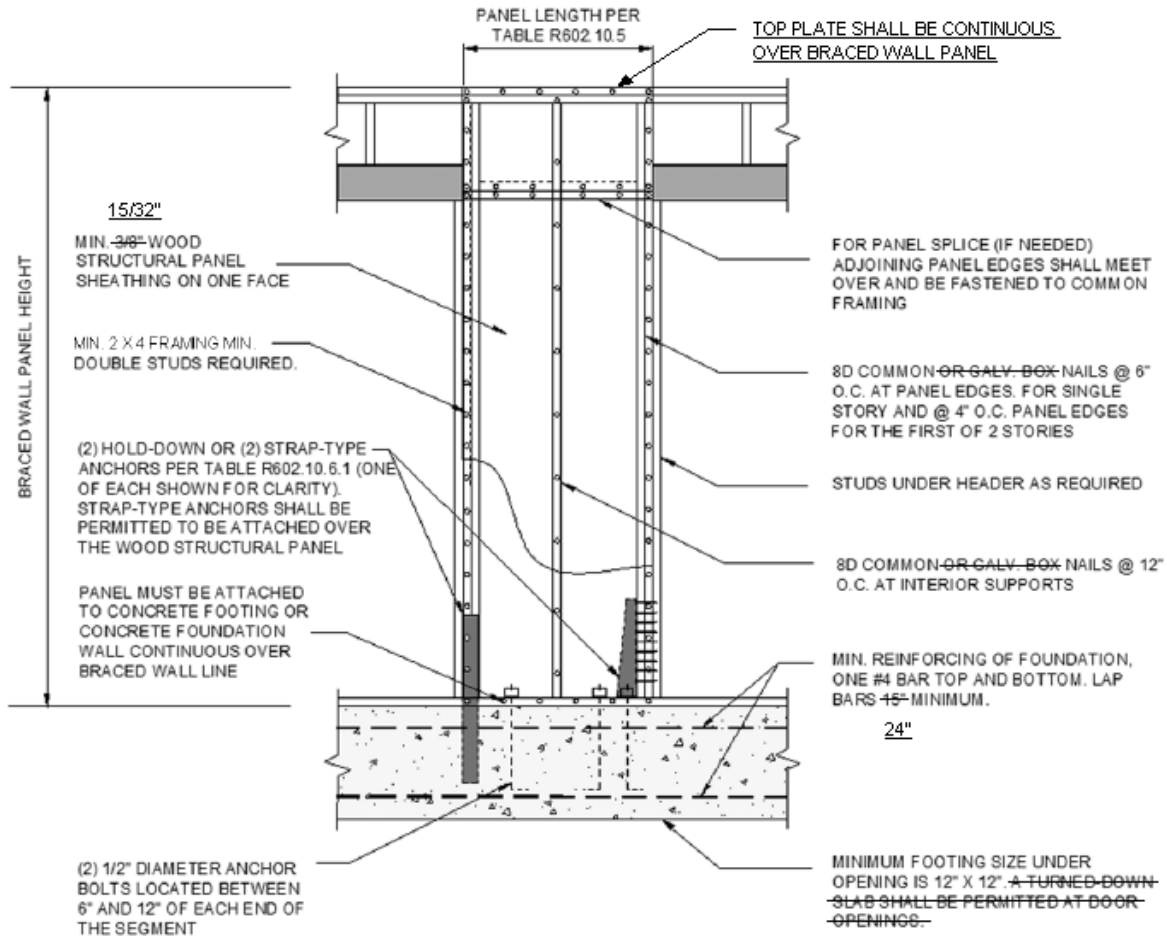


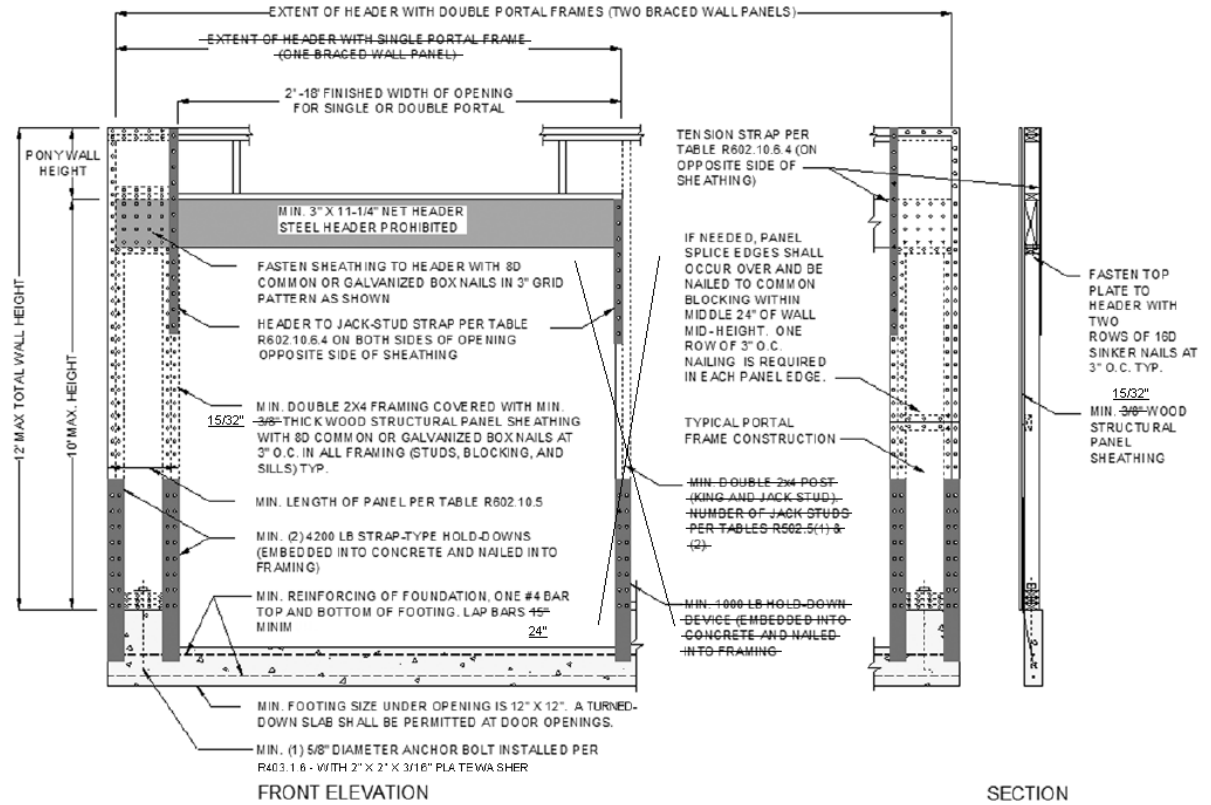
FIGURE R602.10.6.1
METHOD ABW—ALTERNATE BRACED WALL PANEL

SECTION XX.

Figure R602.10.6.2 is hereby amended to read as follows:

FIGURE R602.10.6.2

METHOD PFH – PORTAL FRAME WITH HOLD-DOWNS AT DETACHED GARAGE DOOR OPENINGS

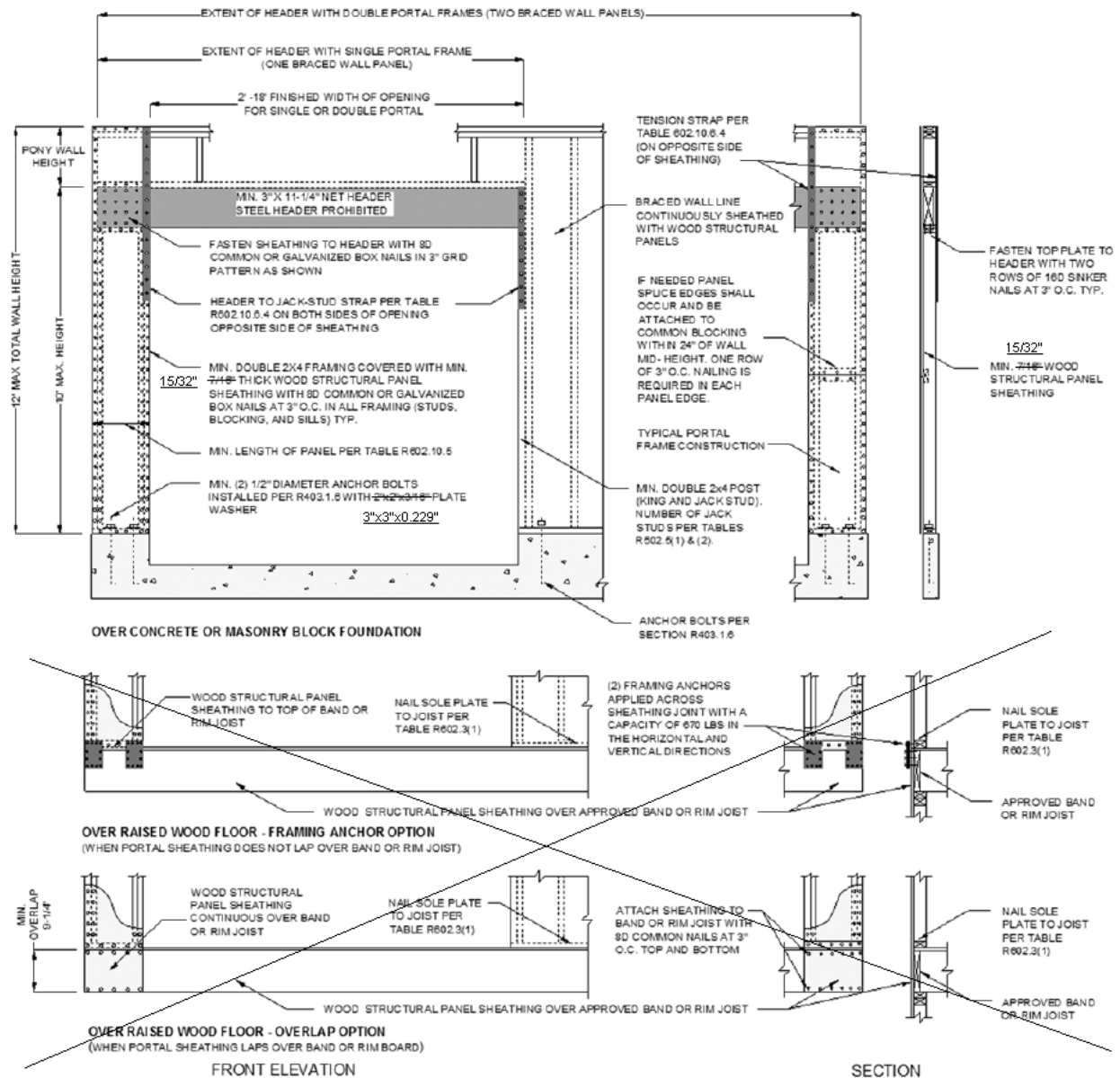


**FIGURE R602.10.6.2
METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS
AT DETACHED GARAGE DOOR OPENINGS**

SECTION XX. Figure R602.10.6.4 is hereby amended to read as follows:

FIGURE R602.10.6.4

METHOD CS-PF – CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

**FIGURE R602.10.6.4
METHOD CS-PF-CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION**

SECTION XX. Section R602.10.9.1 is hereby deleted in its entirety.

~~R602.10.9.1~~ — ~~Braced wall panel support for Seismic Design Category~~

~~D₂. In one-story buildings located in Seismic Design Category D₂, braced wall panels shall be supported on continuous foundations at intervals not exceeding 50 feet (15 240 mm). In two-story buildings located in Seismic Design Category D₂, all braced wall panels shall be supported on continuous foundations.~~

~~Exception: Two-story buildings shall be permitted to have interior braced wall panels supported on continuous foundations at intervals not exceeding 50 feet (15 240 mm) provided that:~~

- ~~1. The height of cripple walls does not exceed 4 feet (1219 mm).~~
- ~~2. First floor braced wall panels are supported on doubled floor joists, continuous blocking or floor beams.~~
- ~~3. The distance between bracing lines does not exceed twice the building width measured parallel to the braced wall line.~~

SECTION XX. Section R606.2.4 is hereby amended to read as follows:

R606.2.4 Parapet walls. Unreinforced solid masonry parapet walls shall not be less than 8 inches (203 mm) thick and their height shall not exceed four times their thickness. Unreinforced hollow unit masonry parapet walls shall be not less than 8 inches (203 mm) thick, and their height shall not exceed three times their thickness. Masonry parapet walls in areas subject to wind loads of 30 pounds per square foot (1.44 kPa) or located in Seismic Design Category D₀, D₁ or D₂, or on townhouses in Seismic Design Category C shall be reinforced in accordance with Section R606.12.

SECTION XX. Section R606.12.2.2.3 is hereby amended to read as follows:

R606.12.2.2.3 Reinforcement requirements for masonry elements.

Masonry elements listed in Section R606.12.2.2 shall be reinforced in either the horizontal or vertical direction as shown in Figure ~~R606.11(2)~~R606.11(3) and in accordance with the following:

1. Horizontal reinforcement. Horizontal joint reinforcement shall consist of ~~at least two longitudinal W1.7 wires spaced not more than 16 inches (406 mm) for walls greater than 4 inches (102 mm) in width and at least one longitudinal W1.7 wire spaced not more than 16 inches (406 mm) for walls not exceeding 4 inches (102 mm) in width; or at least one No. 4 bar spaced not more than 48 inches (1219 mm).~~ Where two longitudinal wires of joint reinforcement are used, the space between these wires shall be the widest that the mortar joint will accommodate. Horizontal reinforcement shall be provided within 16 inches (406 mm) of the top and bottom of these masonry elements.
2. Vertical reinforcement. Vertical reinforcement shall consist of at least one No. 4 bar spaced not more than 48 inches (1219 mm). Vertical reinforcement shall be within ~~16~~8 inches (406mm) of the ends of masonry walls.

SECTION XX. Section R803.2.4 is hereby added to read as follows:

R803.2.4 Openings in horizontal diaphragms.

Openings in horizontal diaphragms shall conform with Section R503.2.4.

SECTION XX. Section R1001.3.1 is hereby amended to read as follows:

R1001.3.1 Vertical reinforcing.

For chimneys up to 40 inches (1016 mm) wide, four No. 4 continuous vertical bars adequately anchored into the concrete foundation shall be placed between wythes of solid masonry or within the cells of hollow unit masonry and grouted in accordance with Section R609. Grout shall be prevented from bonding with the flue liner so that the flue liner is free to move with thermal expansion. For chimneys more than 40 inches (1016 mm) wide, two additional No. 4 vertical bars adequately anchored into the concrete foundation shall be provided for each additional flue incorporated into the chimney or for each additional 40 inches (1016 mm) in width or fraction thereof.